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5 February 2019

Matthew Dodd Quarry Manager Holcim (Australia) Pty Ltd Mt. Shamrock Road

Pakenham, Victoria 3810

Dear Matthew

Mt. Shamrock Quarry - Toomuc Valley Slope Inspection, January 2019

1.0 Introduction and Background

AECOM Services Pty Ltd (AECOM) was requested by Holcim (Australia) Pty Ltd (Holcim) to undertake a visual inspection of the slopes adjacent to the Mt Shamrock Quarry in Pakenham as per the requirements of Mt Shamrock Quarry – Environmental Management Plan (EMP) (Section B, subsection 2.5, Version 3, August 2015).

The EMP requires visual inspection of vegetation planting in previously identified landslip areas, inspection of spring and drainage conditions and monitoring of slope conditions related to movements and instability in the Toomuc Valley.

Luke Clarkson (AECOM – Geotechnical Engineer) completed the inspection on Friday 4th of January 2019 in the company of John Everitt (Holcim), which comprised a walkover survey of the relevant areas as identified in the EMP, which have been inspected on previous occasions by AECOM (Legacy URS). This letter report presents the findings from the annual 2018 inspection.

2.0 Geological Setting

The 1:25,000 scale Pakenham geological map (Geological Survey of Victoria, 1985) indicates that the near surface geology at, and surrounding, the Mount Shamrock Quarry consists of the following units in stratigraphic order from youngest to oldest. Indicative geological overlay is presented in Figure 1 (Attachment 1):

- Qa1 Quaternary Alluvium: Quaternary aged (<0.01 Ma) alluvial sediments consisting of silty sands and clays, deposited along the floor of the Toomuc Valley.
- Tvo Older Volcanics: Miocene aged (23 5 Ma) Older Volcanics of the Thorpedale Volcanic Group, consisting of a single flow of massive, blue/dark grey olivine rich basalts. It is this unit which is currently quarried at Mt Shamrock. It is noted to have a maximum thickness of ~70 m. The Older Volcanic basalt flow is suggested to have extruded into an ancient river valley which resulted in its current vertical and lateral profile. The ancient valley walls have been weathered and eroded to leave the more resistant basalt as a north-south trending ridgeline.
- **Tew Werribee Formation**, fluvial clays and sand deposits, with trace organic material. Where present this unit underlies the Older Volcanics and will be variable in composition and thickness, but typically thin.
- Dgl Lysterfield Granodiorite: Devonian aged Granodiorite. Regionally extensive intrusive unit locally overlain by Werribee Fm sediments (paleo soil) and the basalts of the Older Volcanics(Tvo)
- **S Anderson Creek Siltstone:** The basement rock in the region is Silurian aged siltstone and sandstone.

3.0 History of Landslips in the Toomuc Valley

Old landslips observed on the eastern flanks of the Toomuc Valley in the vicinity of the Mt Shamrock Quarry are likely to be natural occurrences with some possibly pre-dating late 1700's. The Geological Survey of Victoria, 1;25,000 scale 1977-1979 Berwick Generalised Slope Map and accompanying engineering geology report which covers the general area identifies that slopes with gradients greater than 14 degrees formed from tertiary clays or granodioritic materials (Paleo soils) were identified as



exhibiting major old landslides. The report notes the slides as being stable at the time, but suggests they could be remobilised and should be treated as hazardous features.

Remobilisation triggers could be one or a combination of the following:

- Land clearance
- Increased water infiltration and/or groundwater rise due to rainfall or man-made causes
- Slope landform reshaping

The debris from earth slide / slip failures is generally considered to be slow moving, and once it reaches an area of shallower profile, it will typically stop.

4.0 Aerial Photography Identified Historical Landslips

- In preparation for the 2018 site inspection a review of *Google Earth* aerial imagery was undertaken. Using the 2001 aerial imagery,
- Most of the masses identified in the EMP can be seen on the 2001 imagery
- With the benefit of reduced vegetation cover in 2001 it is possible to see that a number of the EMP masses are in fact either:
 - Smaller recent remobilisation of much larger historical landslides that encompass a number of the EMP masses (ie Mass 1, 2, 3, 9 and 17); or
 - Smaller recent remobilisation of a larger historical landslide (ie Mass10, Mass12, Mass13).

The outline of large old landslides, identified from 2001 *Google Earth* aerial imagery, has been included on Figure 1 and Figure 2.

5.0 Summary of discussion with Holcim

The following key observations made by Holcim representatives (Matthew Dodd and John Everitt) since the 2017 inspection are:

- There has been no observed change to any masses, nor any new failures observed;
- Overall the revegetation program is considered to be performing well. The small bare patch in the upper northern corner of Mass 11 observed in the 2017 inspection to not be grassed like elsewhere has been vegetated.
- Changes have been observed at
 - Mass02: Within this mass there is an existing small dam. In the 2017 inspection, it was reported that part of the dam wall has washed out as a result of a recent over topping event. The previous landowner had installed a small agricultural glory hole arrangement to maintain a maximum water level. This system consists of a small tank inserted in the dam with two small diameter flexible poly pipes (<50mm) to transfer water down slope to stock tanks and an open drain located at the lower third of the slope. During the review period, it is understood that Holcim have lowered the gloryhole spillway, and added contingency in the form of an overflow pipe which outlets water to the west in order to prevent embankment overtopping (Attachment 2). A fence is also present to prevent aniomal access.
 - Mass04: During the 2017 review period, a small landslip has occurred within the larger Mass04 area. Remediation works in the form of fencing on all sides to prevent animal access to the area, and revegetation behind the failure/ old backscarp and in the saturated area below the failure has been undertaken. No revegetation of the slump surface with grasses was observed; and
 - No work has been undertaken on or near the other farm dams observed onsite.
- Water levels and water storage volumes within the quarry pit(s) has remained relatively stable over the last 12 months.
- There are a number of long term springs which have been identified across the study area.
 Generally they are positioned in a relatively small elevation band, approximately a third of the way



below the slope crest. Discharge from a number of these springs is being captured and used for stock water and as such it is important to maintain the dams. Anecdotally, discharge from the springs is relatively constant.

AECOM Note: Review of the regional geology and contour information (Figure 1) notes that the identified springs appear to be positioned around elevation 135 m to 165 m AHD which corresponds approximately to the basalt / granodiorite geological boundary.

 All masses identified by the EMP (and reproduced in the in the 2017 annual inspection) are understood to be stable.

6.0 Site Observations

The weather at the time of the inspection was clear with the slopes being generally dry, and vegetated with a good grass cover. The days preceding the site inspection experienced low to negligible rainfall.

The primary objective of the landslip inspections was to determine whether there has been any significant change in the stability of the Toomuc Valley slopes adjacent to the quarry in the period since the previous inspection in October 2017. Any additional geotechnical observations are also to be documented.

In the 2013 inspection report, minor evidence of ground movement was observed in three areas identified as Mass04, Mass09 and Mass19 (Figure 2). The subsequent inspections over 2014, 2016 and 2017 inspections observed that evidence of the original movement is still visible, however there were no indications that the areas had deteriorated further since the 2013 inspection.

All other, visually assessed, EMP identified masses (Figures 1 and 2) did not exhibit evidence of instability. It is understood that a number of masses had previously had limited surveillance as a result of access constraints, however in the 2018 inspection all masses documented were visited. For this reason, additional Plates have been presented in Attachment 2 to provide an understanding of the condition of these areas. As a visual aid, a section has been added to the inspection report to provide an understanding of inspection areas.

Table 1 summarises the visual observations made.



Table 1 Summary of Observations (4 January 2019)

Landslide Reference ID	2018 Visual Observation
	Established vegetation consisting of dense grass, and small trees and shrubs
Mass01	Whilst the slope surface is hummocky, this appears to be associated with historical major landslips and no evidence of recent or ongoing instability was observed.
	General Slope Stability of Mass02 Area
	Overall the area is well vegetated with dense grass cover and small to medium shrubs and trees throughout.
	 Whilst the slope surface is hummocky, this appears to be associated with historical major landslips and no evidence of recent or ongoing instability was observed (exception being existing dam, as discussed below).
	Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area is considered to have remained stable over the last 5 years.
	Two concrete tanks (20 m³) were observed on the downslope, with evident deterioration and removal of grass surrounding this area;
	 Existing Dam (located midway up Mass02) refer Figures 1 and 2. Current water level was not visible due to overgrown vegetation.
	• Evidence that overtopping has occurred (northern end) resulting in washout and scour of the downstream embankment face. It is partially vegetated now. The affected area is (~5 m in width, 1.5 m in height). Apparently the initial event occurred about 4 years
	ago however there is evidence of minor ongoing deterioration.
	 The area is fully vegetated and fenced with tightly spaced tree planting completed in the review period. The dam embankment crest has also been damaged by softening, due to water infiltration, and animal trafficking.
	There is a small intake structure (steel tank) with two small diameter poly pipes to collect dammed water and transfer to lower third
Mass02	of slope. Evidence suggests that these have insufficient capacity to capture water in periods of high water flow into the dam, leading to a high likelihood of further overtopping.
	The gloryhole spillway has been lowered by approximately 200mm (as informed by John Everitt) and a contingency overflow pipe has been added (Attachment 2) which works to prevent embankment overtopping
	A full dam embankment failure may occur at the next overtopping, as the wall has experienced appreciable damage from previous events. The consequences of such a failure would be minor as the asset is isolated from infrastructure such as dwellings, public roads etc. However a full failure would result in rapid discharge of a reasonably significant volume of water and mud which likely would severely damage vegetation and could trigger other remobilisation landslips further down slope. Another consequence would be loss of stock drinking water.
	Remedial Solution Treatments
	The following remedial options have been implemented during the review period:
	Construct a barricade to prevent animal access to the dam crest (fenced) Full supply lovel lowered by 0.2m and complemented by a contingency conflow pine to maintain the full supply lovel.
	 Full supply level lowered by 0.2m and complemented by a contingency overflow pipe to maintain the full supply level It is not anticipated that a hydrological assessment was undertaken in order to determine required pipe size / or chute size.
	4. Reinstate the scoured dam crest with locally won clays (keyed in, placed and compacted in accordance with good dam
	construction practice.
	5. Complete a detailed inspection of the downstream embankment erosion to determine extent. Assuming that it is surficial only, place rock spalls (150-75mm) on face to protect from ongoing erosion.
	*Note: works should be carried out under the direction of an appropriately experienced and qualified dams or geotechnical
	engineer.
	Maintain regular documented inspections of the dam and if conditions deteriorate notify suitably qualified geotechnical engineer for further direction.
	Overall the area is well vegetated with dense grass cover and small to medium shrubs and trees throughout.
Magao	Whilst the slope surface is hummocky, this appears to be associated with historical major landslips and no evidence of recent or
Mass03	 ongoing instability was observed. Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area
	is considered to have remained stable over the last 7 years.
	General Slope Stability of Mass04 Area
	Overall the area is well vegetated with dense grass cover and small to medium shrubs and trees throughout.
	Whilst the slope surface is hummocky, this appears to be associated with historical major landslips and no evidence of recent or ongoing instability was observed (exception being small circular failure of old back scarp, as discussed below).
	Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area
	is considered to have remained stable over the last 5 years. Localised Small Circular Failure of Old Backscarp (refer Attachment 2)
	During review period a small, shallow circular failure of an old backscarp from the larger historical landslide has occurred. The
	slope behind the failure is relatively flat, appeared dry and is vegetated with grass and bushes. There is no evidence of tension cracks behind the failure.
	• The failure measures about 5 m in width and ~1.5 m in height. The slope below slump is damp, likely associated with a spring, with appreciable surface water present.
Mass04	• The circular failure is small in volume and, given that the location is isolated from any downhill infrastructure, is not considered to be a significant geological risk.
	Remedial Solution Treatments
	The following remedial options have been implemented during the review period: 1. Construct a particular to provent animal access to the area to allow regetation to establish, it is currently fenced on all sides at
	1. Construct a barricade to prevent animal access to the area to allow vegetation to establish, it is currently fenced on all sides at approximately a 50m radius.
	2. Revegetate behind the failure / old backscarp with deep root (but small canopy) shrubs / bushes and grasses
	3. Revegetate in the saturated area below the failure with wet tolerant species, ie swampy gums, river gum, boobiellas
	4. Revegetate the slump surface with grasses.
	5. Maintain regular documented inspections and if conditions deteriorate notify suitably qualified geotechnical engineer for further direction.
	It is deemed important to continue to monitor the vegetation of this scarp to ensure that washouts do not inhibit the progress.



Landslide Reference ID	2018 Visual Observation
Mass05	 Overall the area is well vegetated with dense grass cover throughout. Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area is considered to have remained stable over the last 7 years. Minor historical circular failures were observed in three locations: these failures were observed to be associated with exposed
	 (bare) scarps and a change in the natural topography locally downslope with remnant mobilised material. A natural drain has formed through this material to avoid the local high. It is recommended that the bare scarp areas are grassed to mitigate potential of further erosion; The mobilised material is well vegetated. Observation of the exposed scarp suggests that the material is weakly structured, near-
	 Surface soil possibly associated with the Werribee Formation (Attachment 1 for geological map and Attachment 2 for photo of scarp) Upstream drainage channel/ catchment from Mass05 is unobstructed. Embankments are vegetated. Eastern side is bare with an
Mass05 Dam	 observed retreating crest. There is waste (rubbish) blocking the upstream channel to the east; A downstream spillway has formed naturally; and Water was observed 500mm below the crest of the dam, with the crest material softened by cattle traffic. There is a low risk for this
	structure in terms of both likelihood and consequence
Mass06	 Overall the area is well vegetated with dense grass cover throughout. Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area is considered to have remained stable over the last 7 years
Marago Barra	This dam contains a comparatively large volume of water to other dams (surface area 45 m², total depth unknown). The water level was observed at 500mm below the crest; A natural conflow to cost has been identified, which directs water to a dam on an adjacent property dewastroom:
Mass06 Dam	 A natural overflow to east has been identified, which directs water to a dam on an adjacent property downstream; The dam crests are retreating with characteristically erosive material. A visual assessment indicates a low risk of failure; Risk to cattle adjacent to dam, where in some places a 1.5-2m vertical difference was observed due to the retreating crest.
	 Tree planting has been undertaken upstream; There is a minor scarp adjacent to the worn cattle track on the eastern side, suggested erosive driver being the cattle traffic but reiterating the poor structure of the soils;
Mass07	 Historical scarp has been vegetated with no exposed ground or evidence of movement. Overall the area is well vegetated with dense grass cover throughout; Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area
	 o Overall the area is well vegetated with dense grass cover throughout;
Mass08	 There are some historical scarps up to 1m height. These demonstrate weak soil structure, with anticipated minor failures in rain events; The local area immediately below scarp is poorly vegetated; and
	A minor, unrelated scarp was observed upstream.
Mass09	 Overall the area is well vegetated with dense grass cover and small to medium shrubs and trees throughout. Whilst the slope surface is hummocky, this appears to be associated with historical major landslips and no evidence of recent or ongoing instability was observed (exception being small circular failure of old back scarp, as discussed below). Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area
	is considered to have remained stable over the last 7 years.
	 Overall the area is well vegetated with dense grass cover and medium shrubs and trees throughout. Whilst the slope surface is hummocky, this appears to be associated with historical major landslips and no evidence of recent or ongoing instability was observed (exception being small circular failure of old back scarp, as discussed below).
Mass10	 Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area is considered to have remained stable over the last 7 years. Minor grass cover has been removed with bare earth exposed – surficial erosion only.
	 Minor downstream surficial erosion; A drain was observed under road to the south of the mass. No blockage observed at entry/ exit.
	Overall the area is well vegetated with dense grass cover and shrub and tree planting program, on a tight grid, has been established (refer Attachment 2).
Mass11	 Whilst the slope surface is hummocky, this appears to be associated with historical major landslips and no evidence of recent or ongoing instability was observed (exception being small circular failure of old back scarp, as discussed below). A small area in the top north corner of the Mass is bare of grass cover, however this does not appear to represent slope instability
	 and the area has been revegetated since the 2017 inspection. Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area is considered to have remained stable over the last 7 years.
Mass12	 Overall the area is well vegetated with dense grass cover and medium shrubs and trees throughout. Whilst the slope surface is hummocky, this appears to be associated with historical major landslips and no evidence of recent or ongoing instability was observed (exception being small circular failure of old back scarp, as discussed below).
	 Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area is considered to have remained stable over the last 7 years.
	Overall the area is well vegetated with dense grass cover and medium shrubs and trees throughout. While the alone outlood is how modely this grass are to be apposited with historical region landeling and no original and are and are an income.
Mass13	 Whilst the slope surface is hummocky, this appears to be associated with historical major landslips and no evidence of recent or ongoing instability was observed (exception being small circular failure of old back scarp, as discussed below). Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area is considered to have remained stable over the last 7 years.



Landslide Reference ID	2018 Visual Observation
Mass13 Dam	 Refer Attachment 2. Existing Dam Current water level observed to be 500mm below dam embankment crest Dam embankment appears to be in good condition and where water level has exceeded embankment crest it appears to discharge naturally at the northern end of the embankment. The point of discharge has been softened by animal traffic with minor desiccation cracking at crest. Continued uncontrolled discharged of water from an over full dam could result in erosion and scour of the dam wall and whilst the
	consequences of dam failure would be moderate as the asset is isolated from infrastructure such as dwellings, public roads etc. A more detailed assessment should be considered as there is property potentially in line with the dam albeit some distance away. A visual assessment of the dam surface area and anticipated (albeit unknown) depth indicates that should water runout, this would saturate the ground however is not anticipated to detrimentally influence the downstream property. A full failure would result in rapid discharge of a not insignificant volume of water and mud which likely would severely damage vegetation and could trigger other remobilisation landslips further down slope. Another consequence would be loss of stock drinking water.
	 The natural slope at the back of the dam which has experienced a shallow surficial slump has the potential to continue moving and may result in causing an overtopping of the dam if this is to occur. The upstream embankment is steepening/ eroding. Many areas appear to fail locally and deposit material into the dam. The
	pressure of water does not visually appear great enough to destabilise downstream embankment. The primary risk would be degradation of embankment stability as a result of cattle/ ongoing erosion;
	 To maintain the integrity of the dam and prevent damage to the dam wall due to uncontrolled discharged and possible overtopping possible remedial solutions are suggested below. Remedial Solution Treatments
	The following remedial options have been suggested on the basis that, at this point, only small earthmoving plant is considered feasible to mobilise to the area 1. Construct a barricade to prevent animal access to the dam crest and northern area which appears to have been disturbed by
	 Construct a barricade to prevent animal access to the dam crest and northern area which appears to have been disturbed by stock Lower the full supply level to be at least 0.5m below the dam crest and maintain as the full supply level, by construction of a rock lined emergency spillway chute with floor level at design full supply level. The most suitable location is likely towards the north where water appears to be naturally discharging. An engineered solution would ensure that the chute is aligned away from the dam wall (in natural material) so that if discharging water cannot work back and erode / scour the dam embankment). The chute would require rock lining and the outlet would have to be designed so water is dispersed in a manner that does not cause slope instability at or below the point of discharge. A small (say 8t) backhoe would be suitable for this task.
	A hydrological assessment should be undertaken in order to determine required pipe size / or chute size. 3. Remove / trim the failure cutting slope back to undisturbed ground and vegetate with deep-rooted grasses so the excavated surface is stabilised. 4. Install a cut-off swale behind the cut batter on the upslope side of the dam to intercept any slope wash and direct it either
	around the dam or into the dam in a controlled manner. *Note: works should be carried out under the direction of an appropriately experienced and qualified dams or geotechnical
	 engineer. Maintain regular documented inspections of the dam and if conditions deteriorate notify suitably qualified geotechnical engineer for further direction.
Mass14	 Overall the area is well vegetated with dense grass cover and medium shrubs and trees throughout. Whilst the slope surface is hummocky, this appears to be associated with historical major landslips and no evidence of recent or ongoing instability was observed (exception being small circular failure of old back scarp, as discussed below).
	Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area is considered to have remained stable over the last 7 years.
Mass15	 Overall the area is well vegetated with dense grass cover and medium shrubs and trees throughout. Whilst the slope surface is hummocky, this appears to be associated with historical major landslips and no evidence of recent or ongoing instability was observed (exception being small circular failure of old back scarp, as discussed below).
	 Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area is considered to have remained stable over the last 7 years. Minor side-road scarps were observed, anticipated due to cattle traffic;
	John Everitt anecdotally shared that there is a spring present under the large tree at the elevated east of this mass.
Mass16	 Access gained to site but noted that it is important to contact landowner prior to entering. Heavy vegetation in the northern area, predominantly trees but reeds observed as dense nearer the waterway. There is minor side slope erosion visible but anticipated to be related to traffic (both vehicles and cattle). A minor farm dam was observed approximately halfway up the mass. Very heavy vegetation. The water level was observed 1.5m
Mass17	from crest of the dam with minor cattle tracks observed on the crest. • Area is well vegetated with dense grass cover
	 Whilst the slope surface is hummocky, this appears to be associated with historical major landslips and no evidence of recent or ongoing instability was observed. Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area is considered to have remained stable over the last 7 years.
Mass18	Overall the area is well vegetated with dense grass cover and medium shrubs and trees throughout.
Mass19	 Whilst the slope surface is hummocky, this appears to be associated with historical major landslips and no evidence of recent or ongoing instability was observed (exception being small circular failure of old back scarp, as discussed below). Holcim representative indicated that there is no known slope movement in this area over the review period and in general the area
Mass20	is considered to have remained stable over the last 7 years.



Landslide Reference ID	2018 Visual Observation			
Other Areas Visited				
Back scarp of Large Historical Landslide comprising Masses 1, 2, 3, 9 and 17	While the back scarp was not located on this investigation, an intermediate scarp was observed approximately ¾ of the way upslope toward the rear of the scarp.			
	• This area is well vegetated with grasses and trees. The back scarp of the old large landslide is still visible however no evidence of recent or ongoing movement was observed.			



7.0 Conclusions and Recommendations

The project area contains numerous historic landslips that possibly predate the construction of the quarry and occur naturally due to the prevailing conditions relating to the geological setting, slope profile and orientation, and meteorological events.

Typically the landslips observed in the project area present themselves as small remobilisations of old larger landslides. The remobilisation landslips tend to be minor earthflow / earth slide events and debris flows which typically can be triggered by periods of increased rainfall.

No formal risk assessment has been undertaken to date to assess the likelihood and consequence of potential failure or travel distances of deposited material. While it is noted, no properties are currently present in the direct line of travel, there is the possibility of property and asset damage should significant movements occur. Whilst the location and size of the masses identified in the EMP by themselves are unlikely to pose a threat to infrastructure downslope ongoing vigilance, monitoring and revegetation should be undertaken to guard against possible reactivation of larger historic landslides. Areas suggested for further revegetation are described in Table 1.

The observations made during the 2018 landslip inspection suggest no new areas of instability were identified on the slopes surrounding the quarry.

Based on the observations, the following recommendations are suggested:

- Maintain existing regular visual inspection procedures by site personnel (ie maintain the about weekly to fortnightly inspections of each area as part of general operations, with more frequent visits to locations with identified issues). If indications of slope instability are identified, notify a suitably experienced and qualified geotechnical engineer / engineering geologist for further assessment.
- 2. Continue efforts to revegetate the slopes with deep rooted species
- 3. Continue efforts to manage surface water runoff from the natural springs
- 4. Continue to undertake annual inspections during spring and potentially following extended periods of prolonged, intense rainfall should it occur during the review period.

In addition to the landslip monitoring and revegetation recommendations discussed above the site investigation observed five farms dams exhibiting varying levels of degradation, namely:

- Small farm dam within Mass02;
- Moderately sized farm dam to southwest of Mass 05 :
- Larger farm dam on southern side of Mass06;
- Larger farm dam on southern side of Mass13; and
- Small farm dam to south of Mass16.

With insignificant rain in the lead up to the inspection date, the water level in all of these dams was observed at minimum 500mm below the crest. Some remedial solutions for overtopping risk in times of elevated rainfall have been adopted by Holcim, with those not yet addressed remaining in Table 1, aimed at establishing a permanent freeboard and reinstating the dam embankment. Whilst the dams are typically located remotely to existing downslope infrastructure, should failure occur, the potential for downslope damage exists. In the case of the dam at Mass13, more detailed assessment of runout paths should be undertaken to assess the risk, if any, to infrastructure.



Yours faithfully

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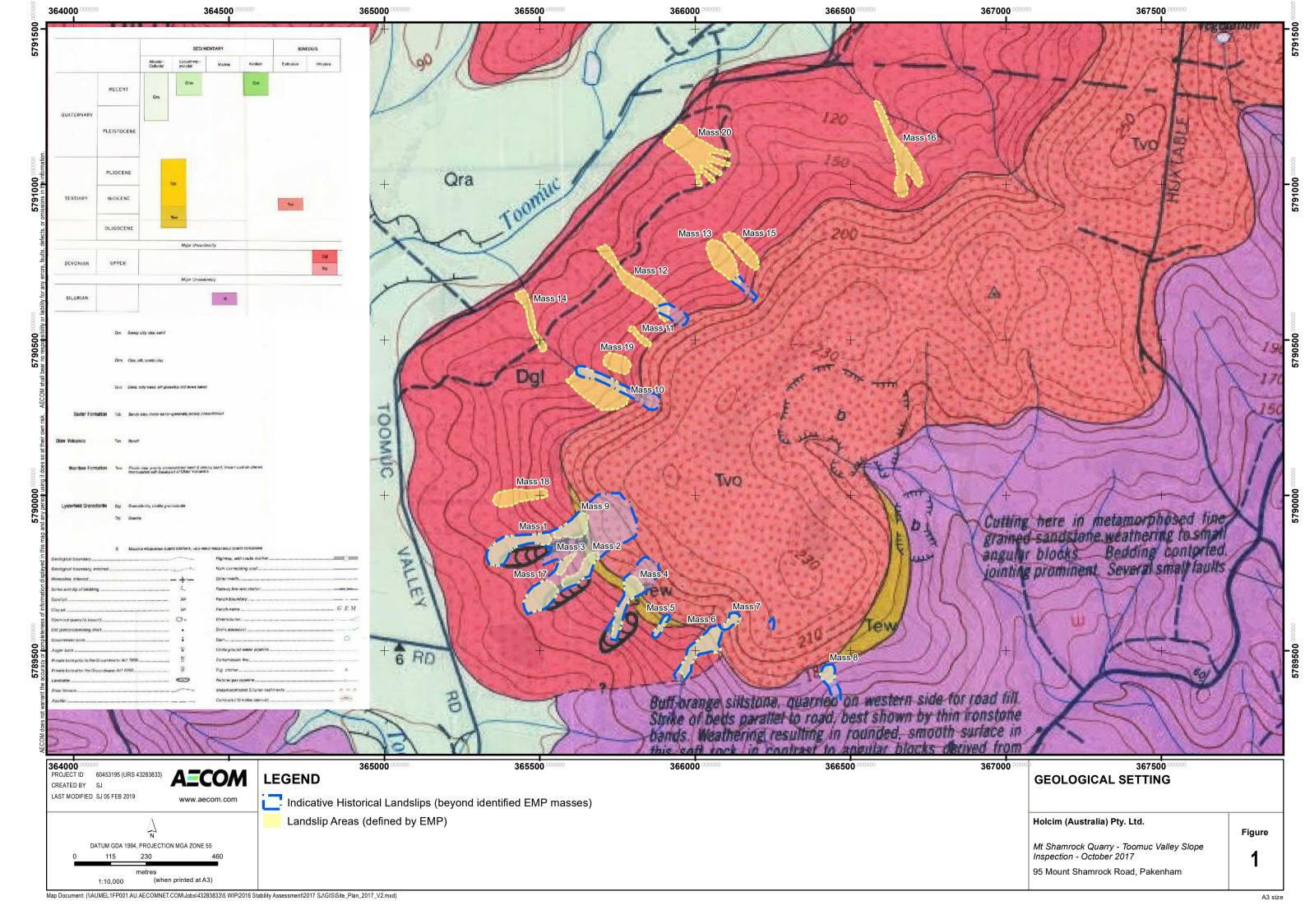
Attachments

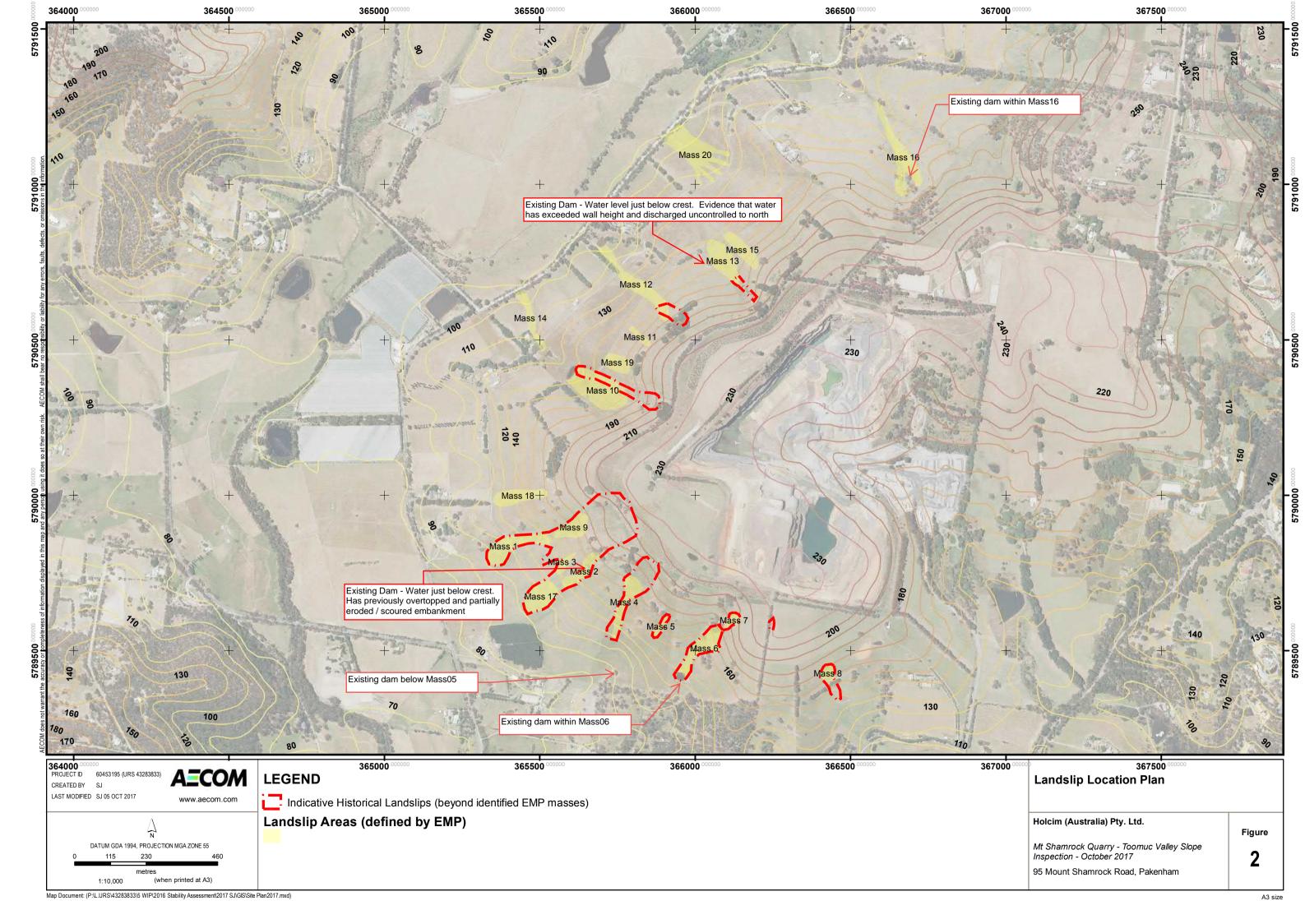
Attachment 1- Figures Attachment 2- Site Photos Chris Huddy Team Leader - Ground Engineering & Tunnelling QNT chris.huddy@aecom.com

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Attachment 1 Figures







Attachment 2 Site Photographs



























Mass05 Dam











Mass06 Dam

























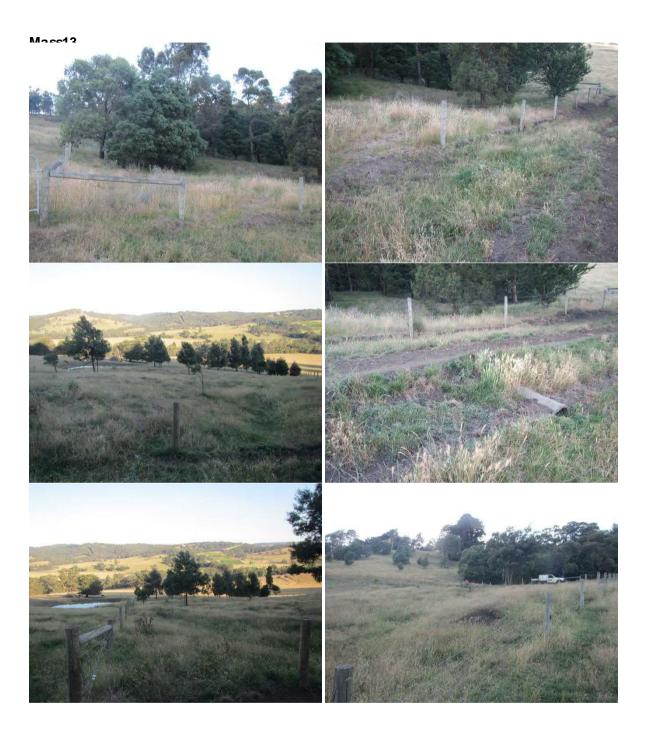


Mass12



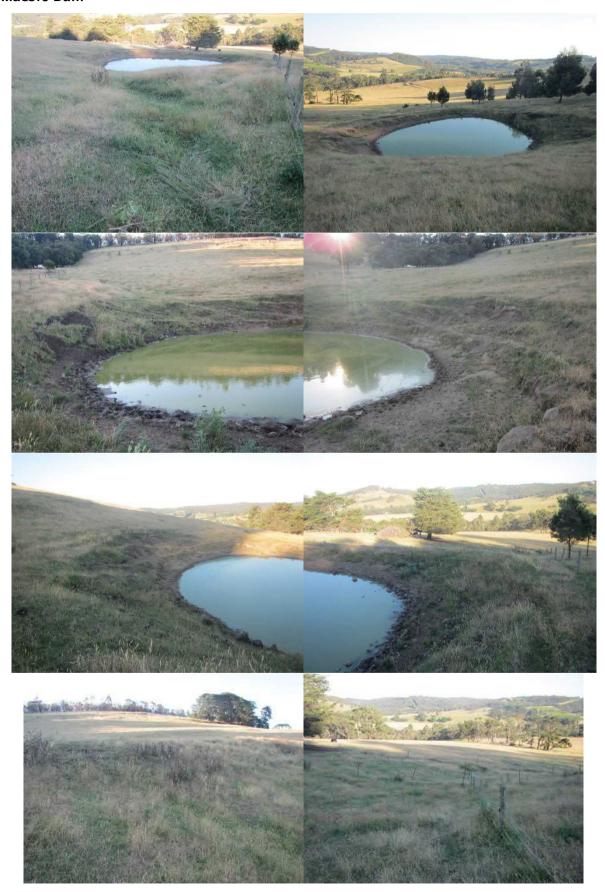








Mass13 Dam





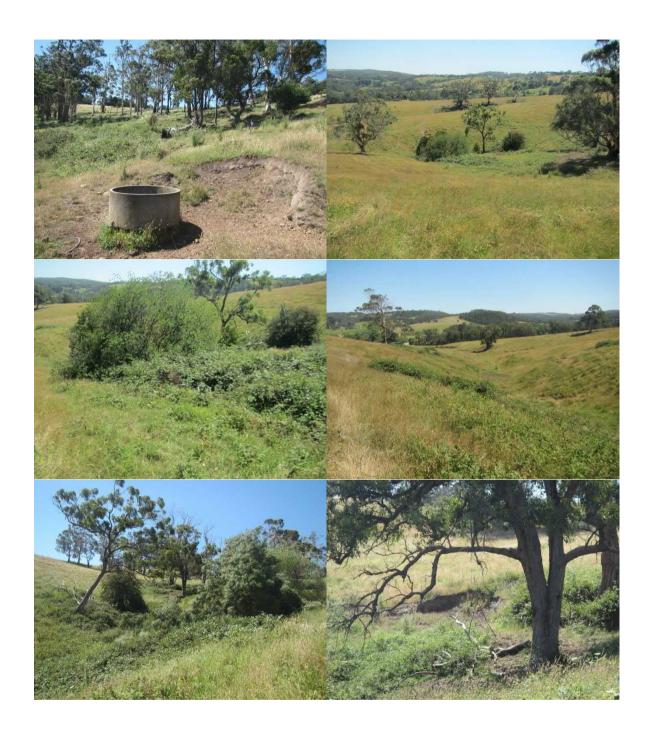




























Mass20







Intermediate scarp to northeast of Mass09, Mass02, and Mass03

